### Advanced FBC: Changes to the Wind Load Provisions of the 2010 Florida Building Code and ASCE 7-10

presented by

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Excerpt										
ASCE 7-05 Section	ASCE 7-10 Section									
Text										
6.1 General	26.1 Procedures									
6.1.1 Scope	26.1.1 Scope									
6.1.2 Allowed Procedures	26.1.2 Procedures									
6.1.3 Wind Pressures Acting on Opposite Faces of Each Building Surface	26.4.3 Wind Pressures Acting on Opposite Faces of Each Building Surface									
-	26.1.2.1 Main Wind Force Resisting System (MWFRS)									
-	26.1.2.2 Components and Cladding									
6.1.4 Minimum Design Wind Loading	-									
	27.4.7 Minimum Design Wind Loads									
6.1.4.1 Main Wind-Force Resisting System	28.4.4 Minimum Design Wind Loading									
	29.8 Minimum Design Wind Loading									
6.1.4.2 Components and Cladding	30.2.2 Minimum Design Wind Pressures									
6.2 Definitions	26.2 Definitions									
6.3 Symbols and Notations	26.3 Symbols and Notations									
	Chapter 28 Part 2: Enclosed Simple Diaphragm Low-Rise Buildings									
6.4 Method 1-Simplified Procedure	28.6 Wind Loads-Main Wind-Force Resisting System									
	Chapter 30 Part 2: Low-rise Buildings (Simplified)									



	Table 28.2-1 Steps to Determine Wind Loads on MWFRS Low-Rise Buildings
	Step 1: Determine risk category of building or other structure, see Table 1.5-1
	Step 2: Determine the basic wind speed, V, for applicable risk category, see Fig. 26.5-1A, B or C
	<ul> <li>Step 3: Determine wind load parameters:</li> <li>&gt; Wind directionality factor, K<sub>d</sub>, see Section 26.6 and Table 26.6-1</li> <li>&gt; Exposure category B, C or D, see Section 26.7</li> <li>&gt; Topographic factor, K<sub>st</sub>, see Section 26.8 and Fig. 26.8-1</li> <li>&gt; Enclosure classification, see Section 26.10</li> <li>&gt; Internal pressure coefficient, (GC<sub>pi</sub>), see Section 26.11 and Table 26.11-1</li> </ul>
	Step 4: Determine velocity pressure exposure coefficient, $K_z$ or $K_h$ , see Table 28.3-1
	Step 5: Determine velocity pressure, $q_z$ or $q_h$ , Eq. 28.3-1
	<b>Step 6:</b> Determine external pressure coefficient, $(GC_p)$ , using Fig. 28.4-1 for flat and gable roofs.
	User Note: See Commentary Fig. C28.4-1 for guidance on hip roofs.
T. Eric Stafford & Associates, LLC	Step 7: Calculate wind pressure, p, from Eq. 28.4-1

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- New model produces more intense hurricanes making landfall than the old model but results in lower wind speeds
- Lower winds associated with the new model for the Holland B parameter.
- Holland B controls the wind-pressure relationship
- Paper describing the statistical model for *B* published in the Journal of Applied Meteorology in 2008.

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		1 1	
Additional Data in Nev	мMo	odel	
	2000	Current	
Parameter	Model	Model	Increase
Number of full scale wind speed traces (with maximum wind	63	245	390%
recorded) used to validate windfield model			
Number of dropsonde profiles used to verify marine	0	650	
boundary layer model			
Number of hurricanes used to develop Holland <i>B</i> model	17	35	100%
Number of landfall hurricanes	167	189	13%
Number of landfall intense hurricanes (defined by pressure)	70	84	20%
Number of hurricanes used to develop filling model	38	57	68%
Number of years of landfall data used to develop model	96	107	11%
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City	V ASCE 7-	V ASCE 7-10	Percent Di Comparat Press	fference in ble Design sures
	05/2007 FBC	(est.)	Exp B Inland	Exp D <sup>2,3</sup> Coastal
Pensacola	140	155	-27%	-12%
Tampa	123	145	-17%	0%
Orlando	110	135	-10%	NA
Miami-Dade <sup>1</sup>	146	175	<b>-1</b> 4% <sup>1</sup>	+3%
Broward <sup>1</sup>	140	170	-12% <sup>1</sup>	+6%
Tallahassee	110	118	-31%	NA
Gainesville	100	125	-7%	NA
Jacksonville	120	125	-35%	-22%



ASCE /-L	0	
	U	
Racio Win	d Speeds	
Dasic will	u specus	
	TABLE C26.5-6	
Design	Wind Speeds: ASCE 7-93 to ASC	CE 7-10
ASCE /-05 Design Wind Speed (3-sec gust in mph)	ASCE /-10 Design Wind Speed (3-sec oust in mph)	ASCE /-93 Design Wind Speed (fastest mile in mnh)
85	110*	71
90	115*	76
100	126	85
105	133	90
110	139	95
120	152	104
130	164	114
140	177	123
145	183	128
150	190	133
150	215	152



#### ASCE 7-10 Exposure Categories 26.7.1 Wind Directions and Sectors For each selected wind direction at which the wind loads are to be determined, the exposure of the building or structure shall be determined for the two upwind sectors extending 45° either side of the selected wind direction. The exposure in these two

sectors shall be determined in accordance with Sections 26.7.2 and 26.7.3, and the exposure whose use would result in the highest wind loads shall be used to represent the winds from that direction.

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### ASCE 7-10 Exposure Categories

26.7.3 6.5.6.3 Exposure Categories.

**Exposure B:** For buildings with a mean roof height of less than or equal to 30 ft (9.1 m), Exposure B shall apply where the ground surface roughness <del>condition</del>, as defined by Surface Roughness B, prevails in the upwind direction for a distance <del>of at least</del> greater than 1,500 ft (457 m). For buildings with a mean roof height greater than 30 ft (9.1 m), Exposure B shall apply where Surface Roughness B prevails in the upwind direction for a distance <u>C of at least greater than</u> 2,600 ft (792 m) or 20 times the height of the building, whichever is greater.

**EXCEPTION:** For buildings whose mean roof height is less than or equal to 30 ft, the upwind distance may be reduced to 1,500 ft.

Exposure C: Exposure C shall apply for all cases where Exposures B or D do not apply.

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### ASCE 7-10 Exposure Categories

**Exposure D:** Exposure D shall apply where the ground surface roughness, as defined by Surface Roughness D, prevails in the upwind direction for a distance greater than 5,000 ft (1,524 m) or 20 times the building height, whichever is greater. Exposure D shall extend into downwind areas of Surface Roughness B or C for a distance of 600 feet (200 m) or 20 times the height of the building, whichever is greater. Exposure D shall also apply where the ground surface roughness immediately upwind of the site is B or C, and the site is within a distance of 600 ft (183 m) or 20 times the building height, whichever is greater, from an exposure D condition as defined in the previous sentence.

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### ASCE 7-10 Protection of Glazed Openings

## • Definitions revised for correlation with ASTM E 1996

**GLAZING, IMPACT RESISTANT:** Glazing that has been shown by testing in accordance with ASTM-E1886 and ASTM E1996 or other approved test methods to withstand the impact of wind-borne test missiles likely to be generated in wind-borne debris regions during design winds. See Section 26.10.3.2.

**IMPACT RESISTANT COVERING:** A covering designed to protect glazing, which has been shown by testing withstood the impact of test missiles. when tested in accordance with ASTM E1886 and ASTM E1996 or other approved test methods to withstand the impact of wind-borne missiles likely to be generated in wind-borne debris regions during design winds.

**IMPACT PROTECTIVE SYSTEM:** Construction that has been shown by testing to withstand the impact of test missiles and that is applied, attached, or locked over exterior glazing. See Section 26.10.3.2.

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### ASCE 7-10 Protection of Glazed Openings **• Wind-borne Debris Regions** WND-BORNE DEBRIS REGIONS: Areas within hurricane prone regions <del>located</del>: where impact protection is required for glazed openings. See Section 26.10.3. • Within 1 mile of the coastal mean high water line where the basic wind speed is equal to or greater than 110 mi/h and in Hawaii, or • In areas where the basic wind speed is equal to or greater than 120 mi/h.

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#### ASCE 7-10 Protection of Glazed Openings. 26.10.3.2 Protection Requirements for Glazed Openings. Glazing in buildings requiring protection shall be protected with an impact protective system impact-resistant covering or shall be impact-resistant glazing, according to the requirements specified in ASTM E1886 and ASTM E1996 or other approved test methods and performance criteria. The levels of impact resistance shall be a function of Missile Levels and Wind Zones specified in ASTM E1886 and ASTM E1996.-Impact protective systems and impact resistant glazing shall be subjected to missile test and cyclic pressure differential tests in accordance with ASTM E1896 as applicable. Testing to demonstrate, compliance with ASTM E1996 shall be in accordance with ASTM E1886. Impact resistant glazing and impact protective systems shall comply with the pass/fail criteria of Section 7 of ASTM E1996 based on the missile required by Table 3 or Table 4 of ASTM E1996.

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### ASCE 7-10 Protection of Glazed Openings

Glazing and impact protective systems in buildings and structures, classified as Risk Category IV in accordance with Section 1.5, shall comply with the "enhanced protection" requirements of Table 3 of ASTM E1996. Glazing and impact protective systems in all other structures shall comply with the "basic protection" requirements of Table 3 of ASTM E1996.

**User Note:** The wind zones that are specified in ASTM E1996 for use in determining the applicable missile size for the impact test, have to be adjusted for use with the wind speed maps of ASCE 7-10 and the corresponding wind borne debris regions, see Section C26.10.3.2.

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### ASCE 7-10 Chapter 29 – Rooftop Equipment

<u>29.5.1</u> 6.5.15.1 ROOFTOP STRUCTURES AND EQUIPMENT FOR BUILDINGS WITH  $h \le 60$  FT (18.3 m).

The lateral force  $F_{h}$ , on rooftop structures and equipment with  $A_{f}$  less than (0.1Bh) located on buildings with a mean roof height,  $h \le 60$  ft (18.3 m) shall be determined from Eq. 29.5-2 6-28. increased by a factor of 1.9. The factor shall be permitted to be reduced linearly from 1.9 to 1.0 as the value of Af is increased from (0.1Bh) to (Bh).

(29.5-2)

 $\underline{F}_{h} = q_{h} (GC_{r}) A_{f} (lb) (N)$ 

where:

 $GC_{1} = 1.9$  for rooftop structures and equipment with  $A_{1}$  less than (0.1Bh).  $GC_{2}$  shall be permitted to be reduced linearly from 1.9 to 1.0 as the value of  $A_{2}$  is increased from (0.1Bh) to (Bh)

 $q_h$  = velocity pressure evaluated at mean roof height of the building

 $A_f =$  vertical projected area of the rooftop structure or equipment on a plane normal to the direction of wind, in ft<sup>2</sup> (m<sup>2</sup>)

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<ul> <li>3) Part 3 is applicable to an enclosed or partially enclosed:</li> <li>Building with h &gt; 60 ft (18.3 m)</li> </ul>	Old analytical method for $h > 60$ ft
<ul> <li>4) Part 4 is a simplified approach and is applicable to an enclosed</li> <li>– Building with h ≤ 160 ft (48.8 m)</li> </ul>	New simplified method for $h \le 160$ ft
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				Coi	mpor	T nents I	Table and Expo	e 30.7 Clao sure	7-2 Iding C	g – Pa	art 4		C V h	C & C 7 = 11 = 15	C 10-12 5-80 1	20 mj ft.	ph
	V (MPH)				110					115					120		
		Load			Zone					Zone					Zone		
h (ft)	Roof Form	Case	1	2	3	4	5	1	2	3	4	5	1	2	3	4	5
	Flat Roof	1	-50.2	-78.8	-107.5	-34.3	-63.0	-54.9	-96.2	-117.5	-37.5	-68.8	-59.8	-93.8	-127.9	-40.9	-74.9
		2	NA	NA	NA	34.3	34.3	NA	NA	NA	37.5	37.5	NA	NA	NA	40.9	40.9
	Gable Roof	1	-37.5	-63.0	-94.7	-40.7	-63.0	-41.0	-68.8	-103.6	-44.5	-68.8	-44.6	-74.9	-112.8	-48.4	-74.9
80	Mansard Roof	2	21.6	21.6	21.6	37.5	34.3	23.6	23.6	23.6	41.0	37.5	25.7	25.7	25.7	44.6	40.9
	HIp Roof	1	-34.3	-59.8	-88.4	-40.7	-63.0	-37.5	-65.3	-96.6	-44.5	-68.8	-40.9	-71.1	-105.2	-48.4	-74.9
		2	21.6	21.6	21.6	37.5	34.3	23.6	23.6	23.6	41.0	37.5	25.7	25.7	25.7	44.6	40.9
	Monoslope Roof	1	-43.9	-56.6	-97.9	-40.7	-63.0	-48.0	-61.9	-107.0	-44.5	-68.8	-52.2	-67.4	-116.5	-48.4	-74.9
		2	18.4	18.4	18.4	37.5	37.5	20.2	20.2	20.2	41.0	41.0	21.9	21.9	21.9	44.6	44.6
	Flat Roof	1	-48.8	-76.7	-104.5	-33.4	-61.2	-53.4	-83.8	-114.2	-36.5	-66.9	-58.1	-91.2	-124.3	-39.7	-72.8
		2	NA	NA	NA	33.4	33.4	NA	NA	NA	36.5	36.5	NA	NA	NA	39.7	39.7
	Gable Roof	1	-36.5	-61.2	-92.1	-39.6	-61.2	-39.9	-66.9	-100.7	-43.2	-66.9	-43.4	-72.8	-109.6	-47.1	-72.8
70	Mansard Roof	2	21.0	21.0	21.0	36.5	33.4	23.0	23.0	23.0	39.9	36.5	25.0	25.0	25.0	43.4	39.7
	HIp Roof	1	-33.4	-58.1	-85.9	-39.6	-61.2	-36.5	-63.5	-93.9	-43.2	-66.9	-39.7	-69.2	-102.3	-47.1	-72.8
		2	21.0	21.0	21.0	36.5	33.4	23.0	23.0	23.0	39.9	36.5	25.0	25.0	25.0	43.4	39.7
	Monoslope Roof	1	-42.7	-55.0	-95.2	-39.6	-61.2	-46.6	-60.1	-104.1	-43.2	-66.9	-50.8	-65.5	-113.3	-47.1	-72.8
		2	17.9	17.9	17.9	36.5	36.5	19.6	19.6	19.6	39.9	39.9	21.3	21.3	21.3	43.4	43.4
						Ра	artic	al ta	ble								
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# 2010 Florida Building Code, Building Wind Speeds

 1609.3 Basic wind speed. The ultimate design wind speed  $V_{ult}$ , in miles per hour, for the development of the wind loads shall be determined by Figures 1609A, 1609B and 1609C. The ultimate design wind speed  $V_{ult}$  for use in the design of Risk Category II buildings and structures shall be obtained from Figure 1609A. The ultimate design wind speed V<sub>ult</sub> for use in the design of Risk Category III and IV buildings and structures shall be obtained from Figure 1609B. The ultimate design wind speed  $V_{\text{ult}}$  for use in the design of Risk Category I buildings and structures shall be obtained from Figure 1609C. The exact location of wind speed lines shall be established by local ordinance using recognized physical landmarks such as major roads, canals, rivers and lake shores wherever possible. 103 © T. Eric Stafford & Associates, LLC



















2010 Protec	Florida Bu ction of Op	uildin Denin	g Co gs	ode, E	Build	ing
	WIND-BORNE SCHEDULE F	DEBRIS PF	609.1.2 ROTECTIOI STRUCTUI	N FASTENI RAL PANEI	NG LS	1
11		FAST	ENER SPA	CING (incl	nes)'*	
	FASTENER TYPE	Panel Span ≤ 2 ft	2 foot < panel Span ≤ 4 feet	4 feet < Panel Span ≤ 6 feet	6 feet < Panel Span ≤ 8 feet	
	#8 Wood screw-based anchor with 2-inch embedment length <sup>3</sup>	16	16	10	8	
	#10 Wood screw-based anchor with 2-inch embedment length <sup>3</sup>	16	16	12	9	
	<sup>1/4</sup> Lag screw-based anchor with 2-inch embedment length <sup>3</sup>	16	16	16	16	113
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### 2010 Florida Building Code, Building Wind Speed Conversions, V<sub>asd</sub>

Table 1507.2.7.1           Classification of Asphalt Shingles										
Classification of Asphalt Shingles										
Maximum Basic Wind V <sub>asd</sub> ASTM D 7158 ASTM D 31										
Speed From Figures										
1609A, B or C										
110	85	D, G or H	A, D or F							
116	90	D, G or H	A, D or F							
129	100	G or H	A, D or F							
142	110	G or H	F							
155	120	G or H	F							
168	130	Н	F							
181	140	Н	F							
194	150	Н	F							
		© T. Eric Stafford & Asso	ociates, LLC							



Section 26.9 shall not be used. Additionally, a simultaneous uplift force shall be applied, given by Equation 29.5-1 in which  $GC_f = 1.5$  and  $A_f$  is replaced by the horizontal projected area,  $A_r$ , of the rooftop structure or equipment. For the uplift force  $GC_f$  shall be permitted to be reduced linearly from 1.5 to 1.0 as the value of  $A_r$  is increased from (0.1BL) to (BL).

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	WI	D STAG	NATION P	TAI	BLE 1609 E ( <i>q<sub>s</sub></i> ) AT	.6.2(1) STANDAI	RD HEIGH	T OF 33	FEET <sup>a</sup>			
BASIC WIND SPEED (mph)	105	110	120	125	130	140	150	160	170	180	190	200
PRESSURE, q <sub>s</sub> (psf)	28.2	31.0	36.9	40.0	43.3	50.2	57.6	65.5	74.0	83.0	92.4	102.4
For 51: 1 Toot = 304.8 mm, 1 a. For basic wind speeds not s	i mile per l	100r = 0.44 $q_r = 0.002$	/ m/s, 1 pc 56 V <sup>2</sup> .	ound per sq	uare foot =	= 47.88 Pa.						
			© T.	Eric Stat	ford & A	Associate	es, LLC				128	

STRUCTURE OR				0.5	CTOP.		
PART THEREOF	DESCRIP	TION	Fac	C <sub>net</sub> FJ			
	Walle-		+ Internal	- Internal	+ Internal	- Internal	
	Windward wall		0.43	0.73	0.11	1.05	
	Leaword well		0.51	0.75	0.02	0.11	
	Eccward wan		-0.51	-0.21	-0.85	0.11	
	Sidewall	Windword	-0.00	-0.55	-0.97	-0.04	
	Parapet wall	Looward		28	1.	28	
1	Boofs:	Leeward	-0	.o.j	-U Partially	anciosad	
	Wind perpendicular to	ridge	+ Internal pressure	- Internal pressure	+ Internal pressure	- Internal pressure	
	Leeward roof or flat	roof	-0.66	-0.35	-0.97	-0.04	
	Windward roof slop	es		2,000			
		Condition 1	-1.09	-0.79	-1.41	-0.47	
	Slope = 2:12 (10°)	Condition 2	-0.28	0.02	-0.60	0.34	
		Condition 1	-0.73	-0.42	-1.04	-0.11	
	Slope = $4:12(18^{\circ})$	Condition 2	-0.05	0.25	-0.37	0.57	
	Slope = 5:12 (23°)	Condition 1	-0.58	-0.28	-0.90	0.04	
1. Main wind-		Condition 2	0.03	0.34	-0.29	0.65	
force-resisting	Slope = $6:12(27^{\circ})$	Condition 1	-0.47	-0.16	-0.78	0.15	
frames and systems	51000 = 0.12 (27)	Condition 2	0.06	0.37	-0.25	0.68	
	Slope = 7:12 (30°)	Condition 1	-0.37	-0.06	-0.68	0.25	
		Condition 2	0.07	0.37	-0.25	0.69	
	Slope = 9:12 (37°)	Condition 1	-0.27	0.04	-0.58	0.35	
	01 10.10 (450)	Condition 2	0.14	0.44	-0.18	0.76	
	Stope = 12:12 (45°)		0.14	0.44	-0.18	0.76	
	Wind parallel to ridge a	and flat roofs	-1.09	-0.79	-1.41	-0.47	
	Nonbuilding Structures:	Chimneys, Tanks and	d Similar Structures		1.0		
				1	n/D	25	
	Source (Wind normal t	o foco)		0.00	1.07	1.52	
	Square (wind normal t	o face)		0.99	1.07	1.55	
	Square (Wind on diago	nal)		0.77	0.84	1.15	
	Hexagonal or Octagona	21		0.81	0.97	1.13	
	Round			0.65	0.81	0.97	
	Open signs and lattice	frameworks		Rati	o of solid to gross	area	
			-	< 0.1	0.1 to 0.29	0.3 to 0.7	
	Flat	© T. Eric	Stafford &	Associates,	LLO.30	1.16	129
	Round			0.87	0.94	1.08	









